

Introductions

- Christi Fisher, PE NRCS
- Craig Odenbach, PE Houston Engineering, Inc.
- Zach Herrmann, PE Houston Engineering Inc.
- LuAnn Kemp Secretary of Pembina County Water Resource District
- Joshua Heuchert Chairman of Pembina County Water Resource District
- Charles Thacker Vice-Chairman of Pembina County Water Resource District
- Don Kemp Manager of Pembina County Water Resource District
- Randall Emanuelson Manager of Pembina County Water Resource District
- Richard Kendall Manager of Pembina County Water Resource District

Agenda



- PL-566 Watershed Rehabilitation Overview
- Background
- Purpose
- Dam and Watershed Plan History
- Dam Safety Concerns
- Flood Prevention Concerns
- Anticipated Alternatives
- Factors Considered
- Public Input



PL-566 Watershed Rehabilitation Overview











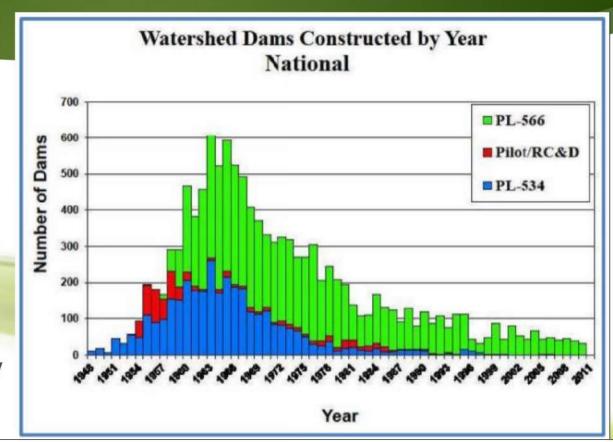
Applicable Agency Authority - Construction

Federal Watershed Protection and Flood Prevention Act

Public Law 78-534 1944

Public Law 83-566 Section 14(a)(1) 1954

11,000+ dams nationally







Applicable Agency Authority - Rehabilitation

2000 Amendment to the Federal Watershed Protection and Flood Prevention Act

Public Law 106-472, Section 313

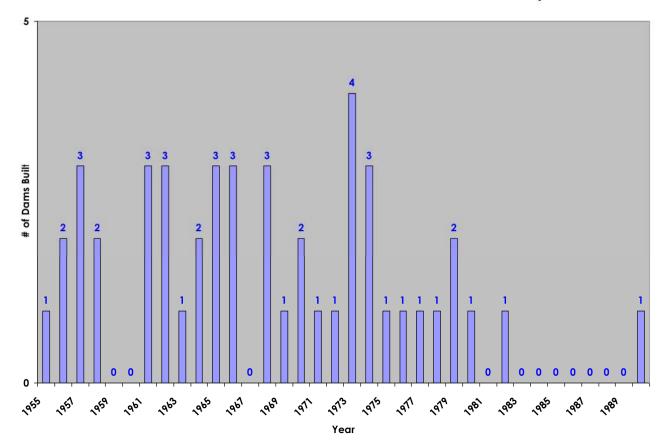
Rehabilitation:

Extend the service life of dams and bring them up to applicable safety and performance standards, or decommission them so they no longer pose a threat to life or property.



Fiscal Year	Program Funding
2000	\$8 M
2001	\$8 M
2002	\$10 M
2003	\$30 M
2004	\$30 M
2005	\$28 M
2006	\$32 M
2007	\$31 M
2008	\$20 M
2009	\$35 M
2010	\$83 M
2011	\$17 M
2012	\$15 M
2013	\$14 M
2014	\$262 M
2015	\$83 M
2016	\$12 M
2017	\$21 M
2018	\$12 M
2019	\$10 M
Total	\$761 M

ND Watershed Dams Built by Year



ND PL-566 Dams

Total = 50

High Hazard (NRCS) = 14

Assessments Complete = 11

Rehabilitation Complete = 1

Planning Underway = 7

2018 Start: Matecjek

2019 Start: Fordville

Larimore

Bylin

Senator Young

Bourbanis

Olson



Planning Team Structure

- Planning Lead: Natural Resources Conservation Service
 - State Conservationist: Mary Podoll
 - State Conservation Engineer: Christi Fisher, PE



- Sponsoring Local Organization: Pembina County Water Resource District
 - Secretary: LuAnn Kemp
 - Chairman: Joshua Heuchert
 - Vice-Chairman: Charles Thacker
 - Manager: Don Kemp
 - Manager: Randall Emanuelson
 - Manager: Richard Kendall
- Planning Technical Lead: Craig Odenbach, PE (Houston Engineering, Inc.)







Planning to Construction

- Planning, typically 2 years
- Design, typically 1-2 years
- Construction, typically 1-2 years
- Complex process, informal and formal public input, regulatory agency involvement
- Local endorsement and support of the plan is key
- Planning requires patience and perseverance



Planning Process-Locally Led

- Purpose and Need for Action
- Data Collection and Resource Inventory
- Development of Alternatives (Structural and non-Structural)
- Evaluation of Alternatives
- Preliminary Supplemental Watershed Plan (EA/EIS)
- Final Supplemental Watershed Plan



Purpose and Need

Purpose: Bring Senator Young, Olson, and Bourbanis Dams into compliance for current federal and state dam safety standards for dam performance, design, and safety while maintaining current flood prevention benefits.

Need:

- Dam Performance, Design, and Safety
- Flood Prevention
- Water Supply



Agency Participation – State & Federal

Federal

- USACE
- USFWS
- FEMA





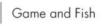


State

- ND State Water Commission
- ND Game & Fish Department
- ND Department of Transportation
- ND State Historical Society
- ND Department of Environmental Quality













Environmental Quality

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Agency Participation - Local

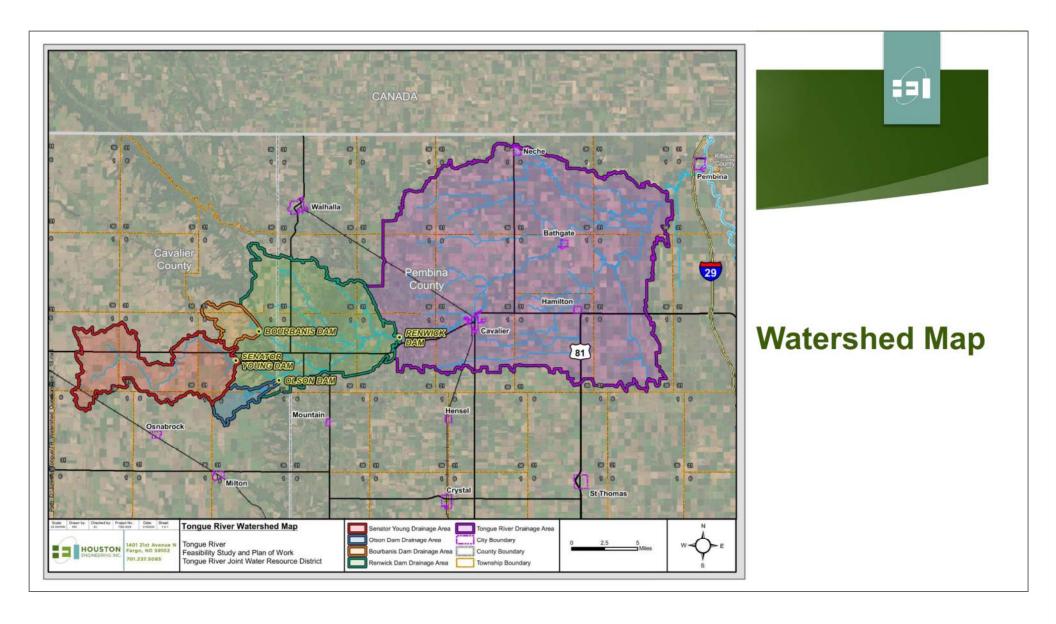
- Tribal Entities
- Pembina & Cavalier County
 - Commissions
 - Water Resource Districts
 - Departments of Emergency Management
 - Soil Conservation Districts
 - Highway Departments



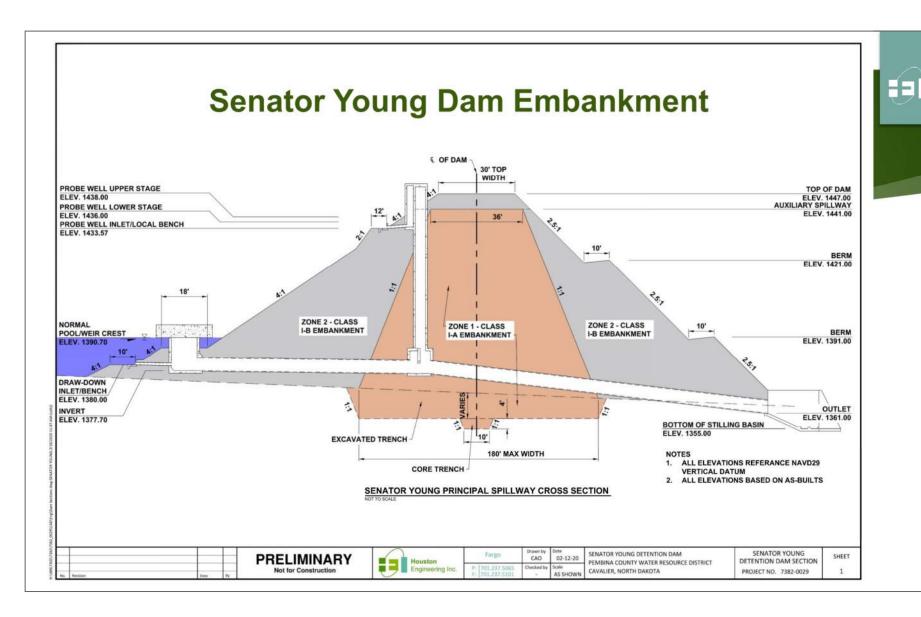


Watershed Work Plan - History

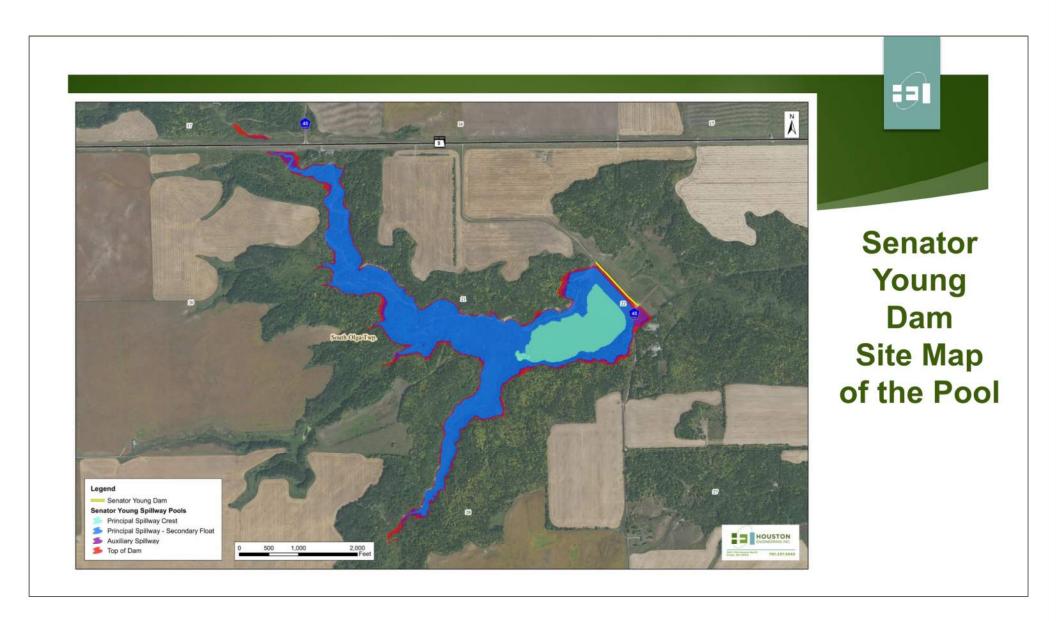
- Part of a Watershed Work Plan for the Tongue River Watershed (SCS, 1954)
- Sponsored by various local Soil Conservation and Flood Control Districts
- Plan included a combination of Land Treatment Measures and Structural Improvements
- A total of eighteen Flood Retarding Structures were planned.
- Other structural improvements included 30 miles of channel improvement and 31.5 miles of floodway construction.
- Annual flood damage estimated at \$734,900 in 1954 Dollars
- Estimated Annual benefits at \$1,828,690 in 1954 Dollars
- Resulted in a Benefit-Cost Ratio of 6.62:1 (SCS, 1954)

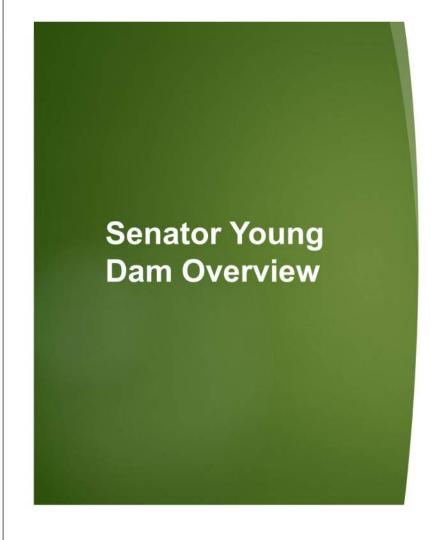










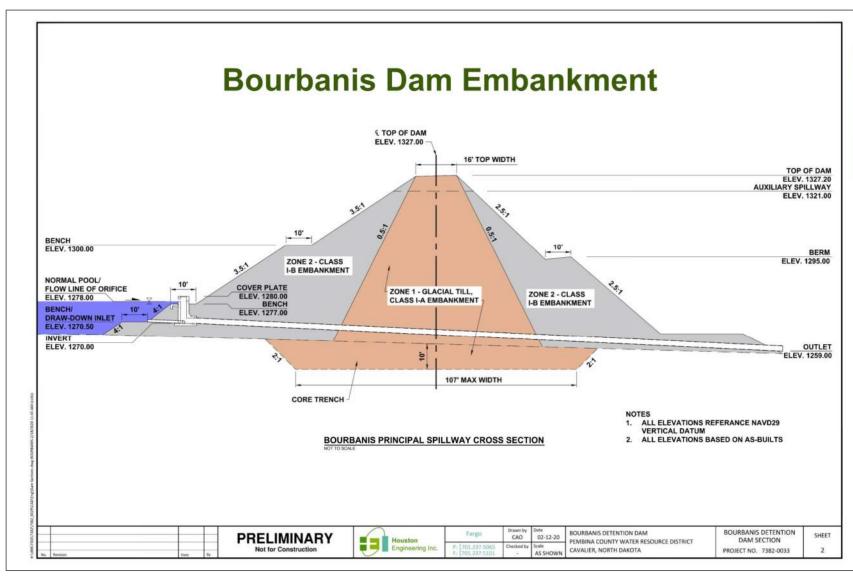


General Data		
Year Design	1955	
Year Constructed	1961	
Purpose(s)	Flood Control, Water Supply	
Original Hazard Classification	Signif	icant
Current Hazard Classification	High	
Design Life	50 ye	ears
Original Design Drainage Area	53.8 sq. mi.	
Revised Drainage Area (Direct) [1]	43.0 sc	q. mi.
Dam Height	81 feet	
Maximum Dam Height	92 fe	eet
Embankment Length	900 1	feet
Embankment Top Width [2]	30 fe	eet
Embankment Upstream Slope	4.0 H:1 V	
Embankment Downstream Slope	2.5 H:1 V	
Critical Elevations	Elevation	(MSL) [3]
Top of Dam	1,447.0	
Auxiliary Spillway Crest	1,441.0	
Principal Spillway Secondary Float	1,436.0	
Principal Spillway Crest	1,390.7	
Principal Spillway Outfall Invert	1,361.0	
Principal Spillway Outlet Channel Invert	1,355.0	
Reference Point	Storage	Surface Area
Top of Dam	5,958 acre - feet	(77
Auxiliary Spillway Crest	5,053 acre - feet	146.7 acres
Principal Spillway Riser Tower Crest	4,345 acre - feet	135.9 acres
Principal Spillway Orifice Inverts	441 acre - feet	38.6 acres

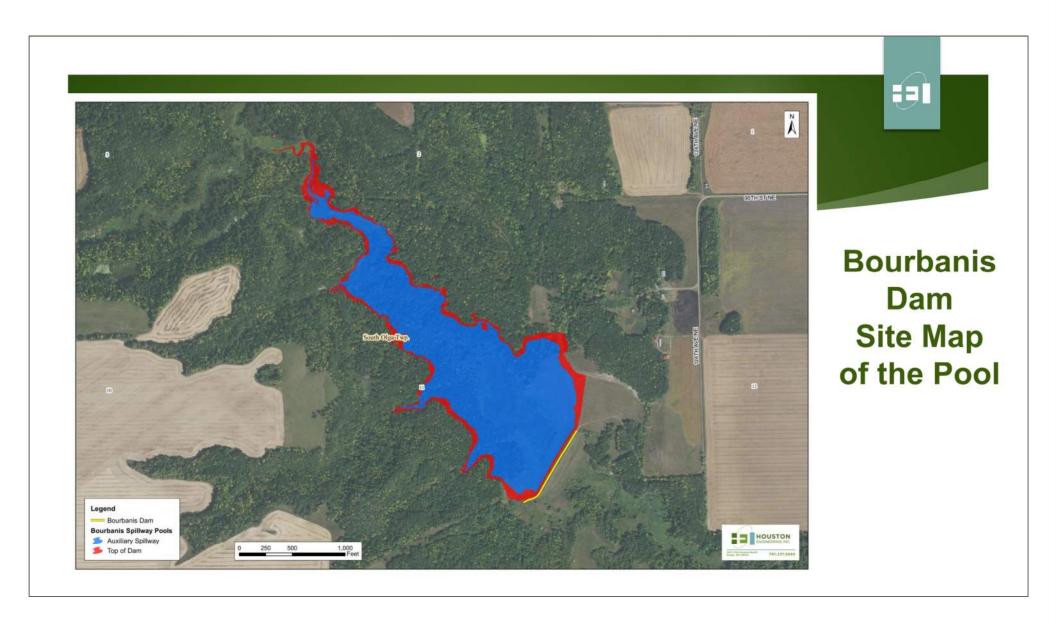
^[1] Revised using available LiDAR information.

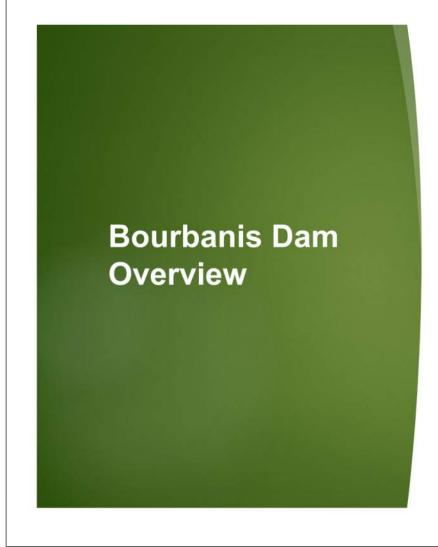
^[2] Includes roadway width of Cavalier County Road 45.
[3] Add 1.21 feet to convert MSL vertical datum to NAVD 1988 vertical datum (approximate).





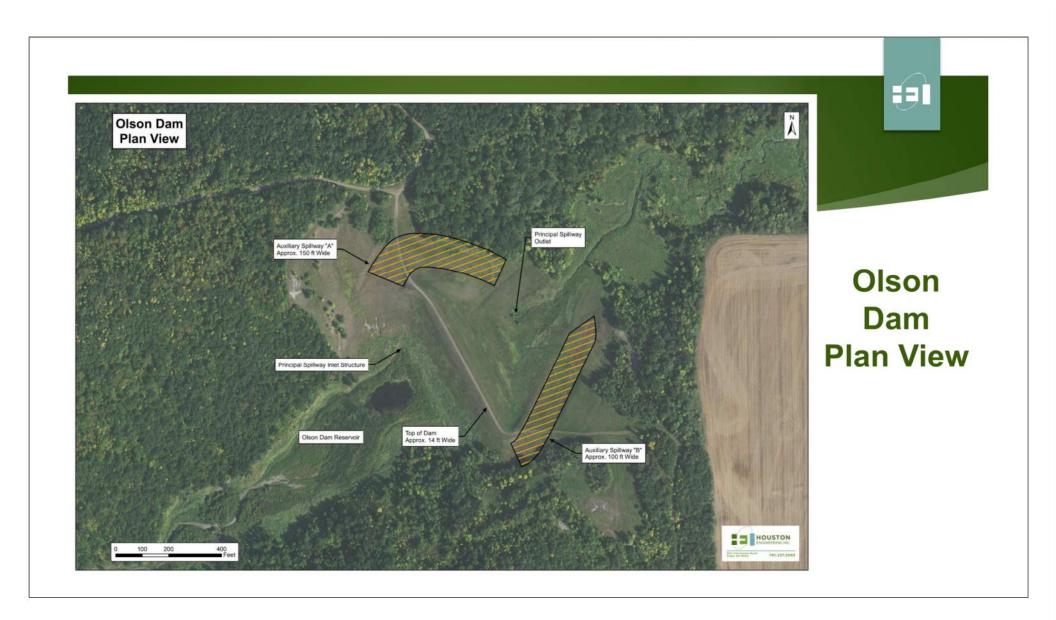


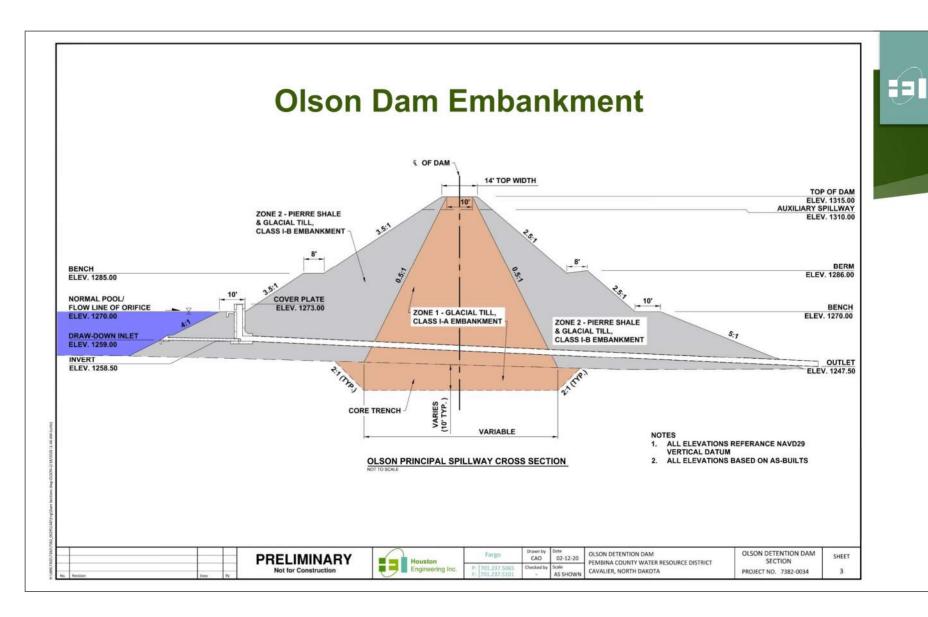




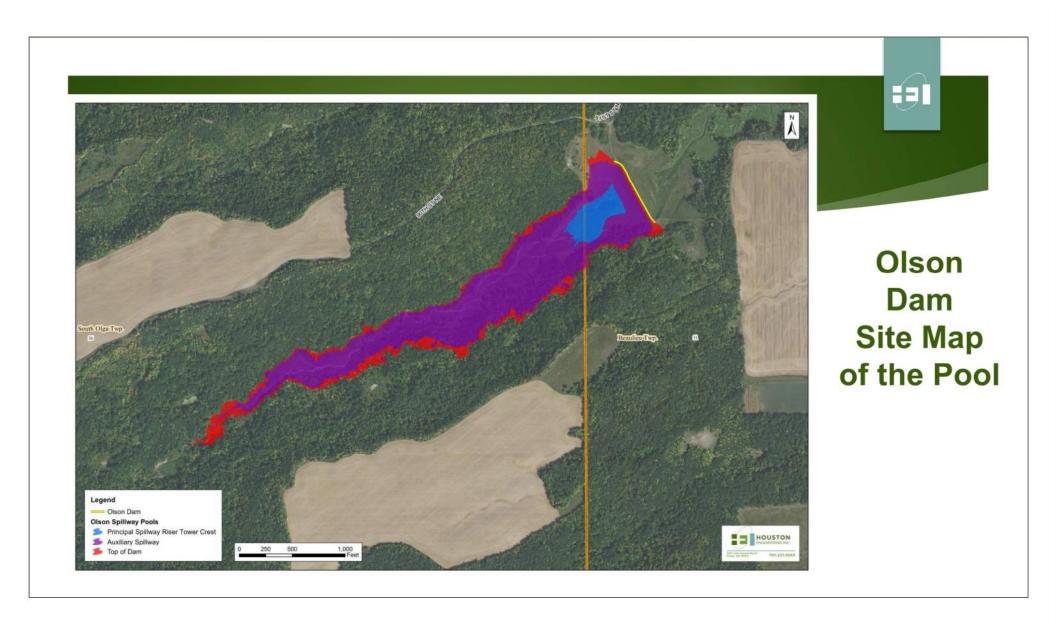
General Data			
Year Design		1955	
Year Constructed		1957	
Purpose(s)	Flood Control		
Original Hazard Classification	Significant		
Current Hazard Classification		High	
Design Life	50) years	
Original Design Drainage Area	8.1	5 sq. mi.	
Revised Drainage Area (Direct) [1]	6.5 sq. mi. (8.6 sq. mi.)		
Dam Height	61 feet 68 feet 745 feet		
Maximum Dam Height			
Embankment Length			
Embankment Top Width	1	16 feet	
Embankment Upstream Slope	3.5 H:1 V 2.5 H:1 V		
Embankment Downstream Slope			
Critical Elevations	Elevati	on (MSL) [3]	
Top of Dam	1,327.0		
Auxiliary Spillway Crest	1,321.0		
Principal Spillway Orifice Invert	1,278.0		
Principal Spillway Top of Riser	1,280.0		
Principal Spillway Outfall Invert	1,259.0		
Principal Spillway Outlet Channel Invert	1,258.0		
Reference Point	Storage	Surface Area	
Top of Dam	1,502 acre - feet	49.5 acres	
Auxiliary Spillway Crest	1,208 acre - feet	49.5 acres	
Principal Spillway	74 acre - feet	9.5 acres	

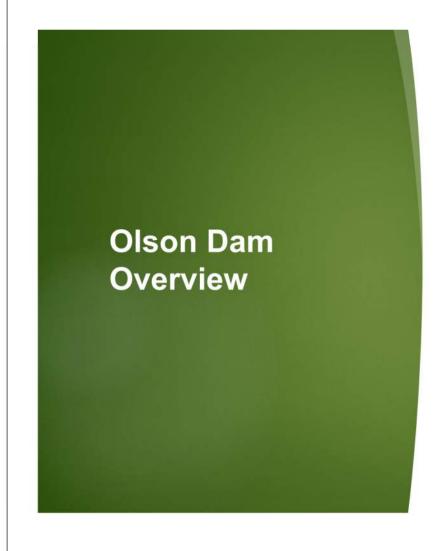
^[1] Revised using available LiDAR information.
[3] Add 1.23 feet to convert MSL vertical datum to NAVD 1988 vertical datum (approximate).











eneral Data		
Year Design	1955	
Year Constructed		1957
Purpose(s)	Flood Control	
Original Hazard Classification	5	Significant
Current Hazard Classification		High
Design Life		50 years
Original Design Drainage Area	5.9 sq. mi.	
Revised Drainage Area (Direct) [1]	6.4 sq. mi.	
Dam Height	65 feet	
Maximum Dam Height	67.7 feet	
Embankment Length		719 feet
Embankment Top Width		14 feet
Embankment Upstream Slope	3.5 H:1 V	
Embankment Downstream Slope	2.5 H:1 V	
Critical Elevations	Eleva	ation (MSL) [3]
Top of Dam		1,315.0
Auxiliary Spillway Crest	1,310.0	
Principal Spillway Orifice Invert	1,270.0	
Principal Spillway Top of Riser	1,273.0	
Principal Spillway Outfall Invert	1,248.5	
Principal Spillway Outlet Channel Invert		1,247.5
Reference Point	Storage	Surface Area
Top of Dam	1,077 acre - feet	
Auxiliary Spillway Crest	861 acre - feet	38.9 acres
Principal Spillway Orifice Invert	61 acre - feet	7.1 acres
Low Level Drawdown	8 acre - feet	14

^[1] Revised using available LiDAR information.
[3] Add 1.28 feet to convert MSL vertical datum to NAVD 1988 vertical datum (approximate).



Downstream Risk

Senator Young Dam*

- Up to 22 potentially inhabitable structures flooded
- One residence located within 2700 feet of the dam

Olson Dam*

- Up to 24 potentially inhabitable structures flooded
- One residence located 4.5 miles downstream of dam

Bourbanis Dam

No breach model completed

*Downstream risk results in a High Hazard Designation

*Breach Analysis From Dam Assessment Report (subject to revision)



Hazard Classification/Dam Safety Standards TR-60 Federal Requirements

Precipitation Depth (Inches)			
Standard	Significant Hazard	High Hazard	
Principal Spillway	P ₅₀	P ₁₀₀	
Auxiliary Spillway	P ₁₀₀ + 0.12 (PMP - P ₁₀₀)	P ₁₀₀ + 0.26 (PMP - P ₁₀₀)	
Freeboard	P ₁₀₀ + 0.40 (PMP - P ₁₀₀)	PMP	

*Probable Maximum Precipitation is being updated and subject to revisions.

** ND Dam Safety Guidelines are being updated.



Hazard Classification/Dam Safety Standards TR-60 Federal Requirements

Dam Assessment Report – Senator Young and Olson Dams

Precipitation Depth (Inches)		
Standard	High Hazard	
Principal Spillway (24-hour / 10-day)	4.8 / 8.8	
Auxiliary Spillway (24-hour)	10.8	
Freeboard (24-hour)	27.1	



Dam Safety - Classification

- Dam Assessment Report Senator Young and Olson Dams do not meet High Hazard Design Standards
 - The Principal Spillways (PS) do not pass the 100-year event without activating the Auxiliary Spillways (AS)
 - The PS's do not evacuate the 100-year storage in 10-days
 - The AS's do not pass the Freeboard Hydrograph without overtopping the dam
- Bourbanis Dam to be evaluated against standards as part of this study



Dam Safety – Other Concerns Senator Young and Olson Dams*

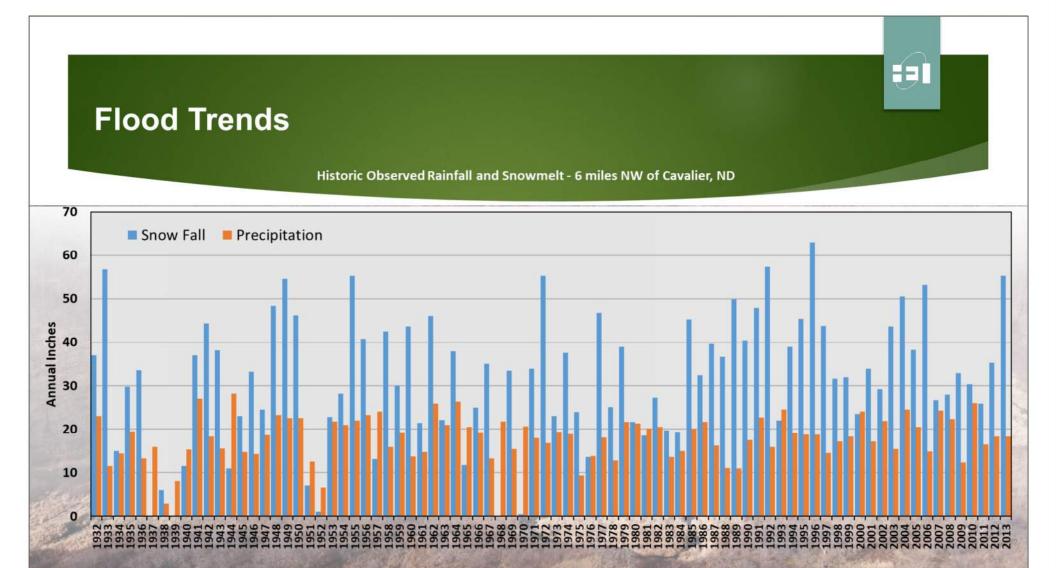
- Soil Stability Analyses show the AS's breaching during a design 24-hour storm
- Existing riser structure is non-standard
- Irregular downstream slope
- No filter diaphragm used along the PS conduit
- Observed seepage areas

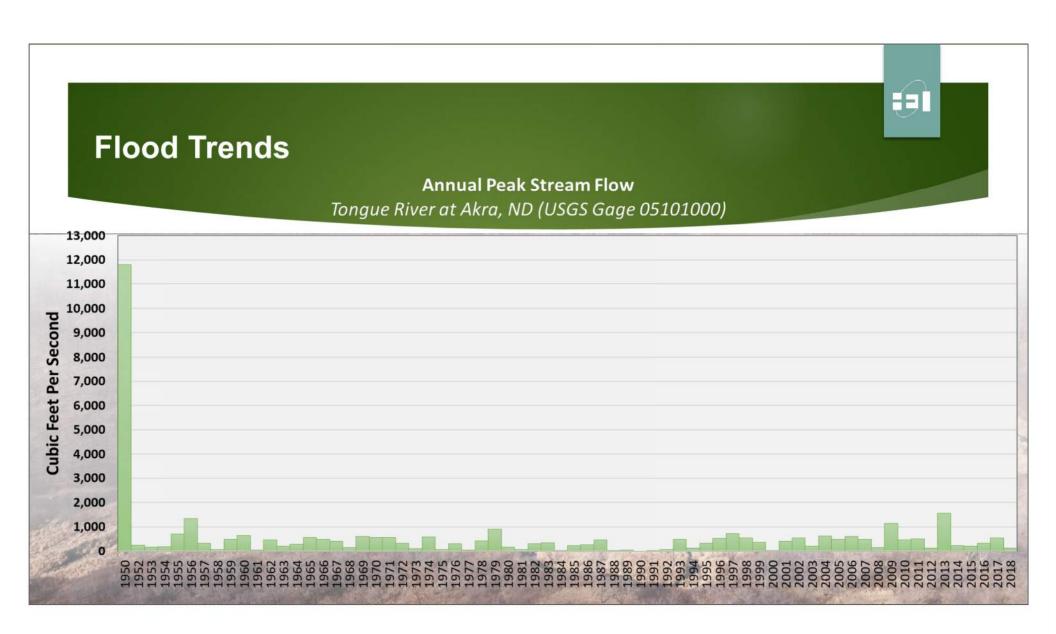
*Bourbanis Dam to be evaluated as part of this study effort

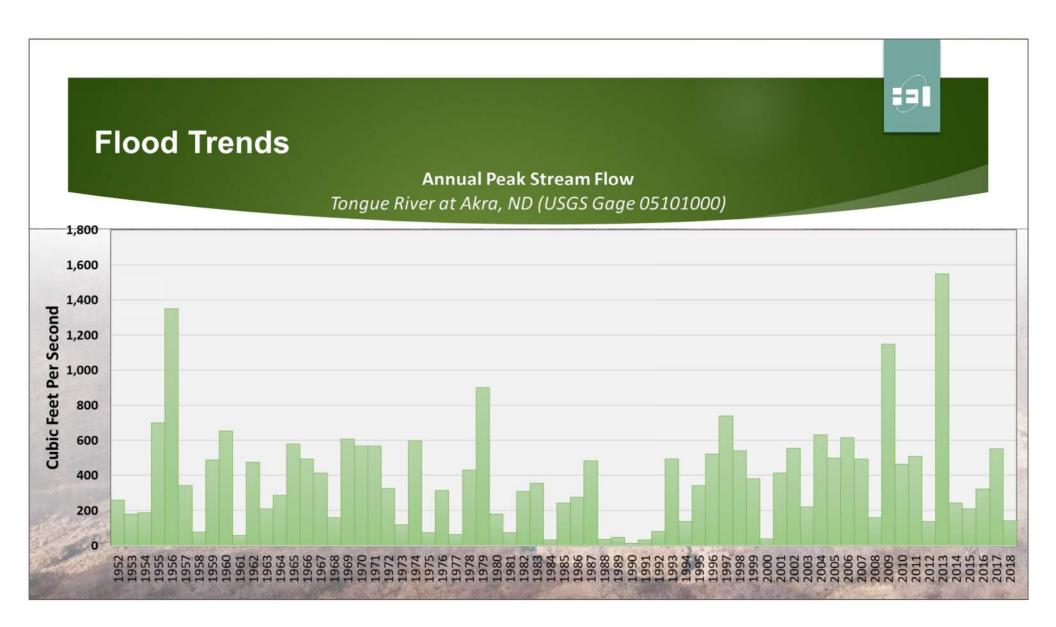


Flood Prevention

- Agriculture delayed planting, prevented planting, crop damage, reduced yields, overland erosion, etc.
- Infrastructure road overtopping, culvert washouts, road embankment scour, critical utilities, etc.
- Structural Inhabited & uninhabited buildings
- Public Safety Impacted transportation systems (possible critical access routes)





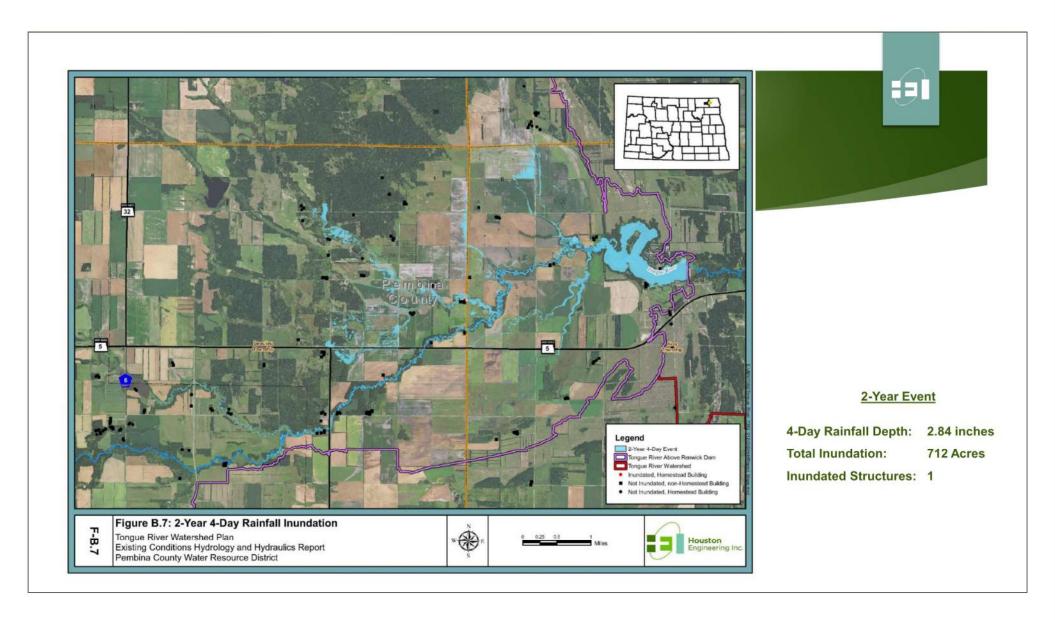


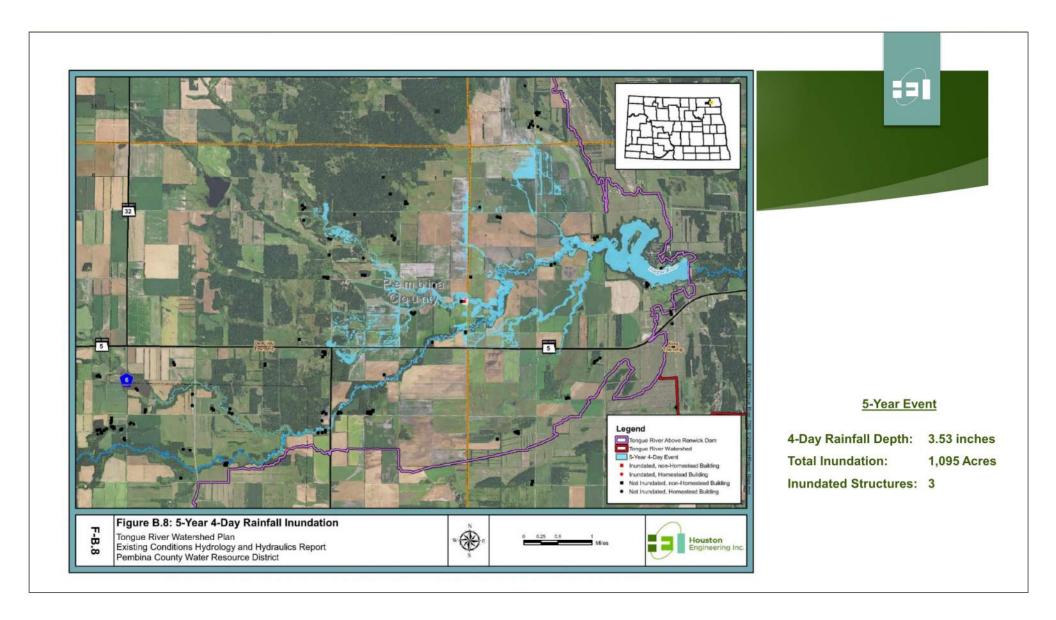


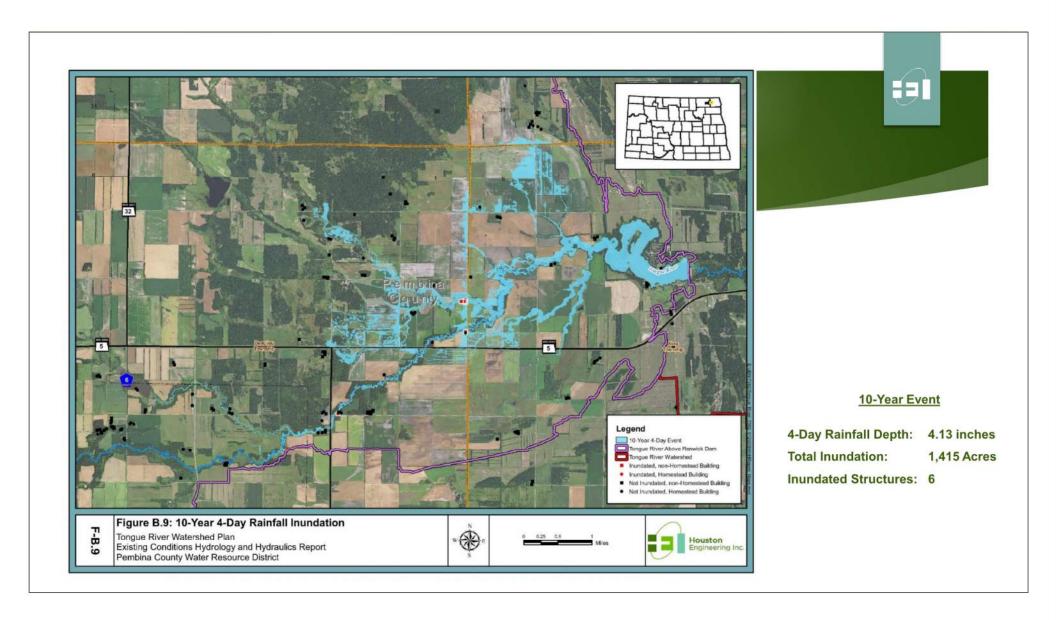
Hydrology and Hydraulics

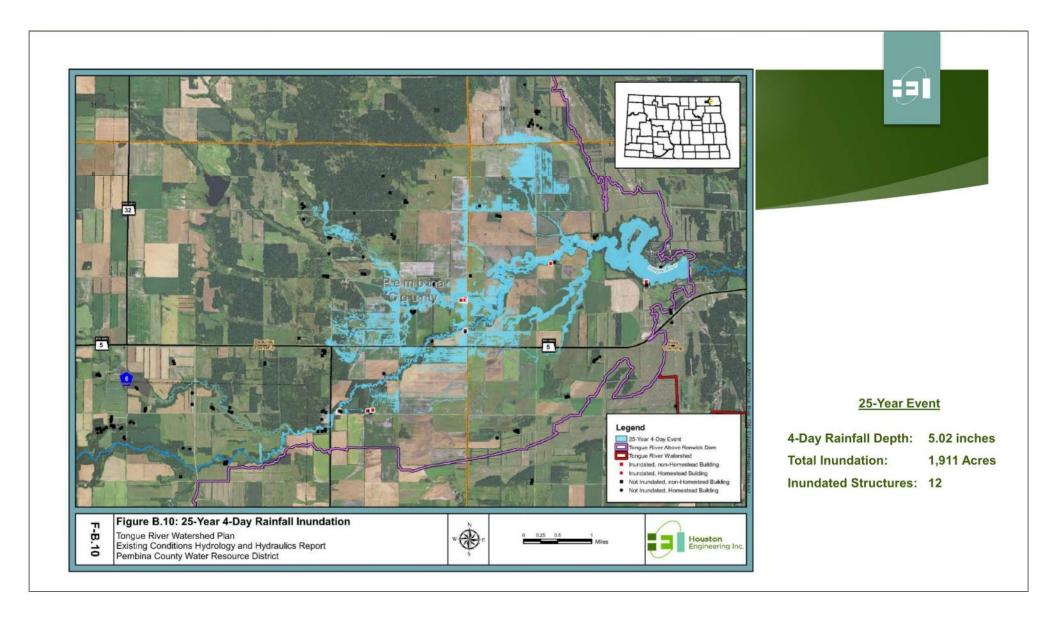
- HEC-HMS hydrologic model developed to estimate flood flow and timing characteristics for the Tongue River Watershed
- HEC-RAS unsteady state hydraulic model was developed estimate flood elevations using HEC-HMS results
- Hydrology and hydraulic models were calibrated to historic rainfall events
 - June 2002
 - May 2013
- Synthetic rainfall events were modeled (Source Atlas 14)
- Model used to determine inundated acreages, duration of flooding, and potential damages to structures.
- Review of results determine the 4-day duration rainfall is the most damaging rainfall event.

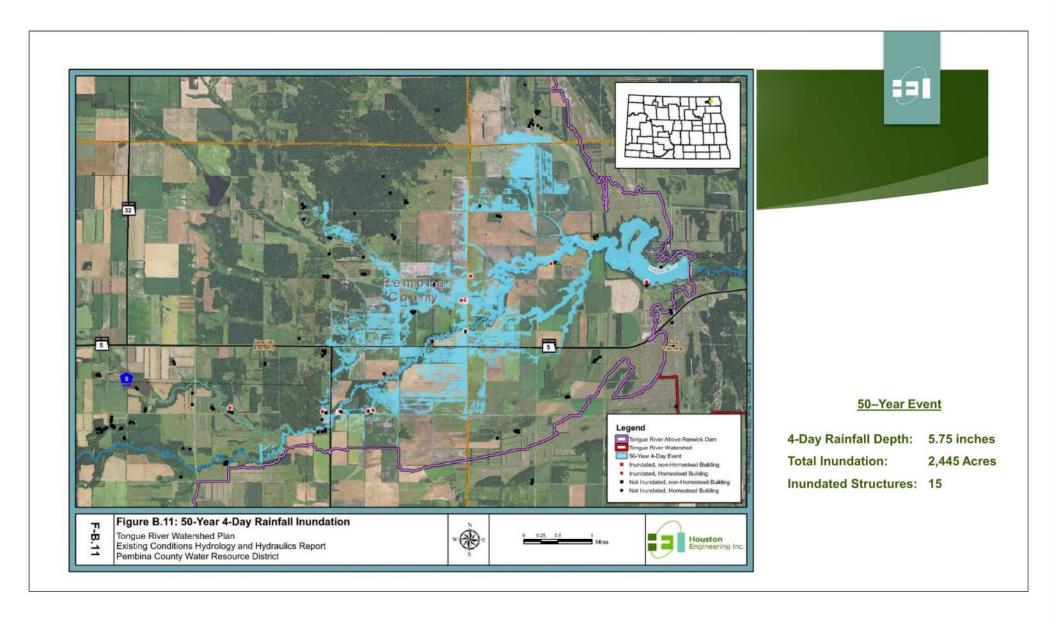
Return Period	24-Hour Rainfall (in.)	4-Day Rainfall (in.)	10-Day Rainfall (in.)
2-year	2.2	2.8	3.7
5-year	2.9	3.5	4.4
10-year	3.4	4.1	5.1
25-year	4.2	5.0	6.0
50-year	4.9	5.8	6.7
100-year	5.6	6.5	7.5

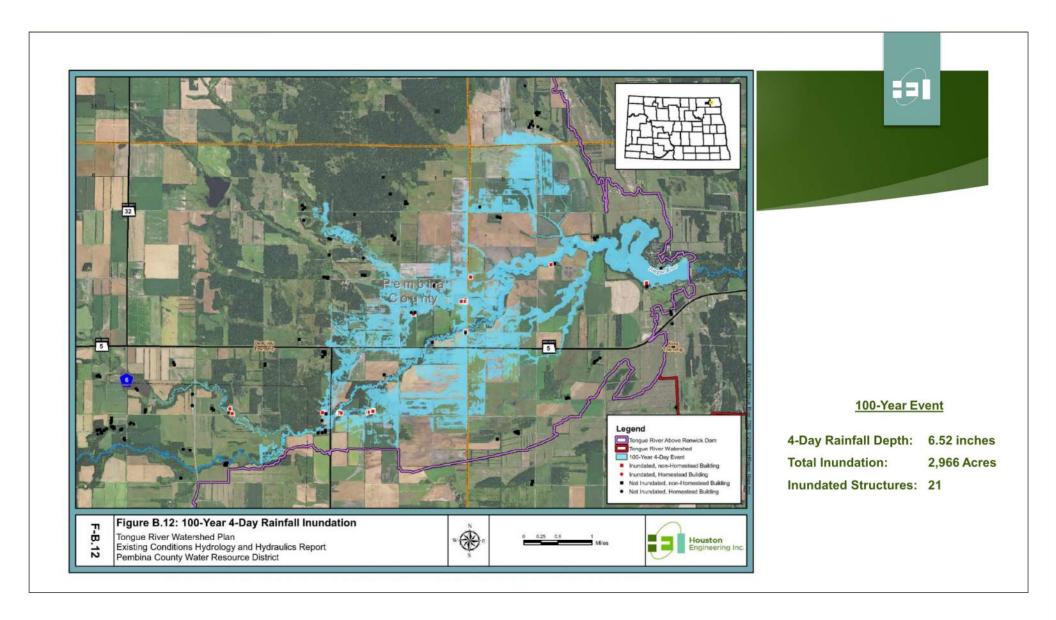


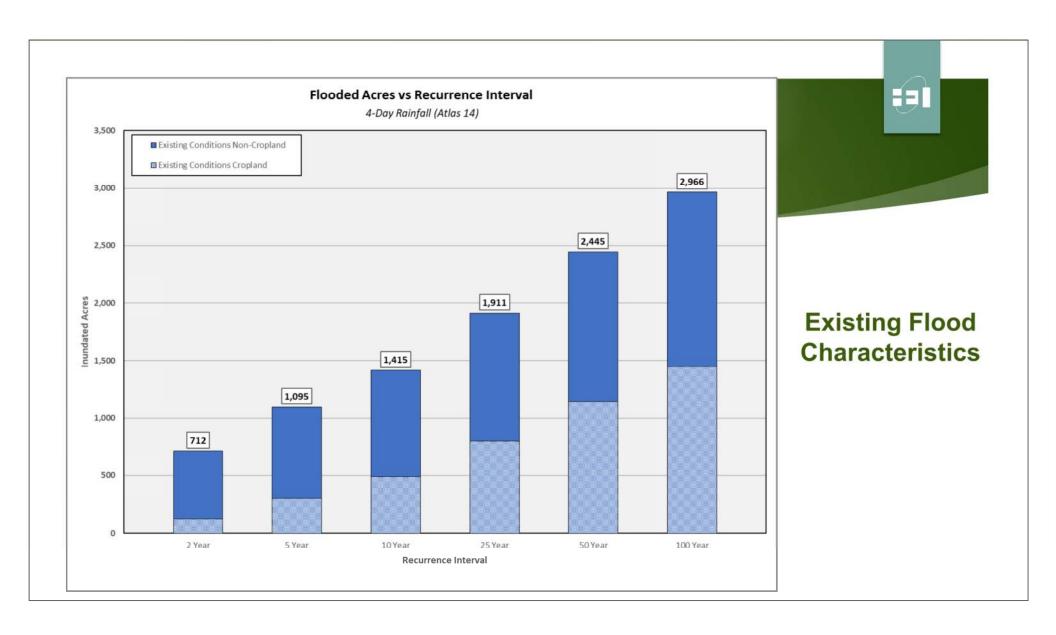


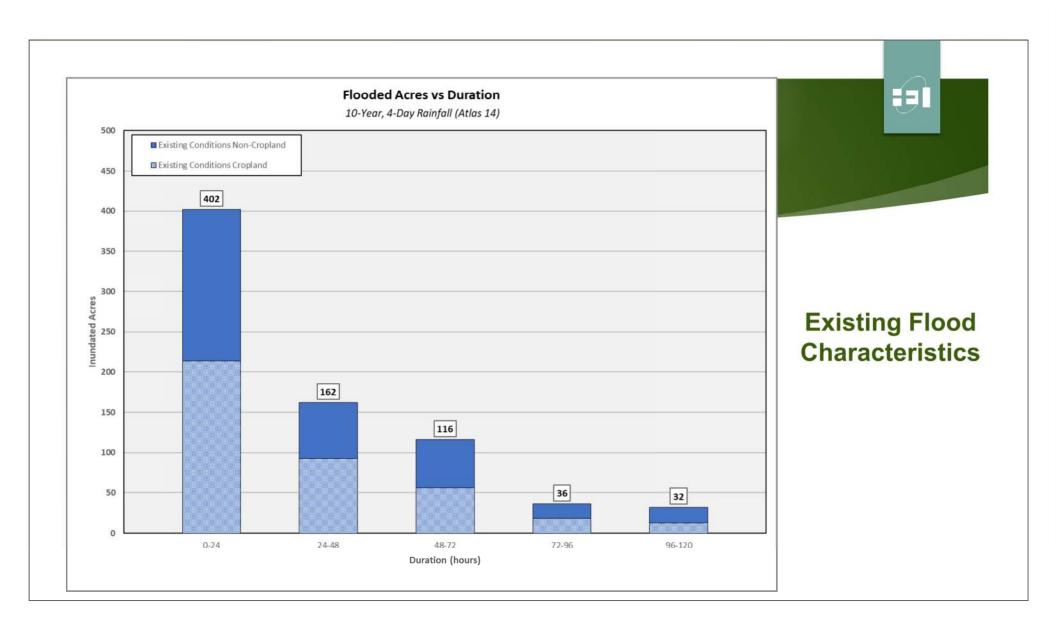












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Alternatives

- No Action
- Decommissioning
- Rehabilitation to High Hazard Criteria
- Remove Downstream Flood Risk and Rehabilitate to Reduced Hazard Classification





Factors Considered

- Technical Feasibility
- Economic Efficiency
- Public Health and Safety
- Environmental
- Social Considerations
- Cultural and Historic Resources
- Other Unique Factors



Planning Process Timeline

ш	Purpose and Need for Action; Public Participation	July, 2020
	Data Collection and Resource Inventory	_March, 2021
	Development of Alternatives (Structural and non-Structural)	July, 2021
	Evaluation of Alternatives_	February, 2022
m	Preliminary Supplemental Watershed Plan (EA/EIS)	April, 2022
	Final Supplemental Watershed Plan	July, 2022

